

Phone: Fax:
E-mail:

-----Operational Analysis-----

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: AM Peak
Freeway/Direction: I-75 NB
From/To: Dearborn Exit/Springwells Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	5141	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1353	v
Trucks and buses	16	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.926	
Driver population factor, fp	1.00	
Flow rate, vp	1461	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	1461	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	26.6	pc/mi/ln

Level of service, LOS

D

Overall results are not computed when free-flow speed is less than 55 mph.

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----- Operational Analysis -----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: AM Peak
 Freeway/Direction: I-75 NB
 From/To: Springwells Exit/Plaza Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Flow Inputs and Adjustments -----

Volume, V	4739	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1247	v
Trucks and buses	17	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.922	
Driver population factor, fp	1.00	
Flow rate, vp	1353	pc/h/ln

----- Speed Inputs and Adjustments -----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

----- LOS and Performance Measures -----

Flow rate, vp	1353	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	24.6	pc/mi/ln

Level of service, LOS

C

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-----Operational Analysis-----

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: AM Peak
Freeway/Direction: I-75 NB
From/To: Plaza Exit/Livernois Ent.
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	4345	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1143	v
Trucks and buses	11	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.948	
Driver population factor, fp	1.00	
Flow rate, vp	1206	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	1206	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	21.9	pc/mi/ln

Level of service, LOS

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Date Performed: 8/22/2007
Analysis Time Period: AM Peak
Freeway/Direction: I-75 NB
From/To: Livernois Ent./Junction Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	4652	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1224	v
Trucks and buses	11	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.948	
Driver population factor, fp	1.00	
Flow rate, vp	1033	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	1033	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	18.8	pc/mi/ln

Level of service, LOS

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Date Performed: 8/22/2007
Analysis Time Period: AM Peak
Freeway/Direction: I-75 NB
From/To: Junction Exit/Plaza Ent.
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	4510	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1187	v
Trucks and buses	11	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.948	
Driver population factor, fp	1.00	
Flow rate, vp	1252	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	1252	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	22.8	pc/mi/ln

Level of service, LOS

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Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: AM Peak
Freeway/Direction: I-75 NB
From/To: Plaza Ent./Clark Ent.
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	5203	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1369	v
Trucks and buses	13	%
Recreational vehicles	0	%
Terrain type:	Grade	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.939	
Driver population factor, fp	1.00	
Flow rate, vp	1458	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	1458	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	26.5	pc/mi/ln

Level of service, LOS

D

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-----Operational Analysis-----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 11/26/2007
 Analysis Time Period: AM Peak
 Freeway/Direction: I-75 NB
 From/To: Clark Ent./Grand Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	5532	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1456	v
Trucks and buses	14	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.935	
Driver population factor, fp	1.00	
Flow rate, vp	1246	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	1246	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	22.7	pc/mi/ln

Level of service, LOS

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 Date Performed: 11/26/2007
 Analysis Time Period: AM Peak
 Freeway/Direction: I-75 NB
 From/To: Grand Exit/I-96 WB Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	5513	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1451	v
Trucks and buses	14	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.935	
Driver population factor, fp	1.00	
Flow rate, vp	1242	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h

Urban Freeway

-----LOS and Performance Measures-----

Flow rate, vp	1242	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	22.6	pc/mi/ln

Level of service, LOS

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-----Operational Analysis-----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: Midday Peak
 Freeway/Direction: I-75 NB
 From/To: Dearborn Exit/Springwells Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	2751	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	724	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	814	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	814	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	14.8	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

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-----Operational Analysis-----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: Midday Peak
 Freeway/Direction: I-75 NB
 From/To: Springwells Exit/Plaza Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	2477	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	652	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	733	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	733	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	13.3	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

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-----Operational Analysis-----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: Midday Peak
 Freeway/Direction: I-75 NB
 From/To: Plaza Exit/Livernois Ent.
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	1769	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	466	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	524	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	524	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	9.5	pc/mi/ln

Level of service, LOS

A

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-----Operational Analysis-----

Analyst: CH
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Date Performed: 8/22/2007
Analysis Time Period: Midday Peak
Freeway/Direction: I-75 NB
From/To: Livernois Ent./Junction Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	1880	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	495	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	445	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	445	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	8.1	pc/mi/ln

Level of service, LOS

A

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Date Performed: 8/22/2007
Analysis Time Period: Midday Peak
Freeway/Direction: I-75 NB
From/To: Junction Exit/Plaza Ent.
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

----- Flow Inputs and Adjustments -----

Volume, V	1830	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	482	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	542	pc/h/ln

----- Speed Inputs and Adjustments -----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

----- LOS and Performance Measures -----

Flow rate, vp	542	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	9.9	pc/mi/ln

Level of service, LOS

A

Overall results are not computed when free-flow speed is less than 55 mph.

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E-mail:

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-----Operational Analysis-----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: Midday Peak
 Freeway/Direction: I-75 NB
 From/To: Plaza Ent./Clark Ent.
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	1984	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	522	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	587	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	587	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	10.7	pc/mi/ln

Level of service, LOS

A

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Operational Analysis

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 11/26/2007
 Analysis Time Period: Midday Peak
 Freeway/Direction: I-75 NB
 From/To: Clark Ent./Grand Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

Flow Inputs and Adjustments

Volume, V	2379	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	626	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	563	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	563	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	10.2	pc/mi/ln

Level of service, LOS

A

Overall results are not computed when free-flow speed is less than 55 mph.

Phone:
E-mail:

Fax:

-----Operational Analysis-----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 11/26/2007
 Analysis Time Period: Midday Peak
 Freeway/Direction: I-75 NB
 From/To: Grand Exit/I-96 WB Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	2135	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	562	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	506	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	506	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	9.2	pc/mi/ln

Level of service, LOS

A

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:
E-mail:

-----Operational Analysis-----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: PM Peak
 Freeway/Direction: I-75 NB
 From/To: Dearborn Exit/Springwells Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	3391	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	892	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	1004	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	1004	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	18.3	pc/mi/ln

Level of service, LOS

C

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:
E-mail:

Operational Analysis

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: PM Peak
Freeway/Direction: I-75 NB
From/To: Springwells Exit/Plaza Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

Flow Inputs and Adjustments

Volume, V	3116	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	820	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	923	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	923	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	16.8	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:
E-mail:

-----Operational Analysis-----

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: PM Peak
Freeway/Direction: I-75 NB
From/To: Plaza Exit/Livernois Ent.
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	2147	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	565	v
Trucks and buses	18	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.917	
Driver population factor, fp	1.00	
Flow rate, vp	616	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	616	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	11.2	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: _____ Fax: _____
 E-mail: _____

 Operational Analysis

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: PM Peak
 Freeway/Direction: I-75 NB
 From/To: Livernois Ent./Junction Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

 Flow Inputs and Adjustments

Volume, V	2280	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	600	v
Trucks and buses	17	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.922	
Driver population factor, fp	1.00	
Flow rate, vp	521	pc/h/ln

 Speed Inputs and Adjustments

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

 LOS and Performance Measures

Flow rate, vp	521	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	9.5	pc/mi/ln

Level of service, LOS

A

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:
E-mail:

----- Operational Analysis -----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: PM Peak
 Freeway/Direction: I-75 NB
 From/To: Junction Exit/Plaza Ent.
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Flow Inputs and Adjustments -----

Volume, V	2218	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	584	v
Trucks and buses	18	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.917	
Driver population factor, fp	1.00	
Flow rate, vp	636	pc/h/ln

----- Speed Inputs and Adjustments -----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

----- LOS and Performance Measures -----

Flow rate, vp	636	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	11.6	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone:
E-mail:

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Operational Analysis

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: PM Peak
Freeway/Direction: I-75 NB
From/To: Plaza Ent./Clark Ent.
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

Flow Inputs and Adjustments

Volume, V	2340	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	616	v
Trucks and buses	21	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.905	
Driver population factor, fp	1.00	
Flow rate, vp	680	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

LOS and Performance Measures

Flow rate, vp	680	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	12.4	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone:
E-mail:

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-----Operational Analysis-----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 11/26/2007
 Analysis Time Period: PM Peak
 Freeway/Direction: I-75 NB
 From/To: Clark Ent./Grand Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	3049	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	802	v
Trucks and buses	19	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.913	
Driver population factor, fp	1.00	
Flow rate, vp	703	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h

Urban Freeway

-----LOS and Performance Measures-----

Flow rate, vp	703	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	12.8	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:
E-mail:

Operational Analysis

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 11/26/2007
 Analysis Time Period: PM Peak
 Freeway/Direction: I-75 NB
 From/To: Grand Exit/I-96 WB Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

Flow Inputs and Adjustments

Volume, V	2845	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	749	v
Trucks and buses	17	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.922	
Driver population factor, fp	1.00	
Flow rate, vp	650	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	650	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	11.8	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone:
E-mail:

Fax:

-----Operational Analysis-----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: AM Peak
 Freeway/Direction: I-75 SB
 From/To: Ambassador Ent./Grand Ent.
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	3386	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	891	v
Trucks and buses	14	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.935	
Driver population factor, fp	1.00	
Flow rate, vp	763	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h

Urban Freeway

-----LOS and Performance Measures-----

Flow rate, vp	763	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	13.9	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:
E-mail:

Operational Analysis

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: AM Peak
Freeway/Direction: I-75 SB
From/To: Grand Ent./Clark Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

Flow Inputs and Adjustments

Volume, V	3414	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	898	v
Trucks and buses	14	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.935	
Driver population factor, fp	1.00	
Flow rate, vp	641	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	6	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	641	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	6	
Density, D	11.7	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:
E-mail:

----- Operational Analysis -----

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: AM Peak
Freeway/Direction: I-75 SB
From/To: Clark Exit/Plaza Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

----- Flow Inputs and Adjustments -----

Volume, V	2812	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	740	v
Trucks and buses	17	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.922	
Driver population factor, fp	1.00	
Flow rate, vp	642	pc/h/ln

----- Speed Inputs and Adjustments -----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

----- LOS and Performance Measures -----

Flow rate, vp	642	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	11.7	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone:
E-mail:

Fax:

Operational Analysis

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: AM Peak
 Freeway/Direction: I-75 SB
 From/To: Plaza Exit/Junction Ent.
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

Flow Inputs and Adjustments

Volume, V	2596	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	683	v
Trucks and buses	14	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.935	
Driver population factor, fp	1.00	
Flow rate, vp	731	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

LOS and Performance Measures

Flow rate, vp	731	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	13.3	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone:
E-mail:

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Operational Analysis

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: AM Peak
Freeway/Direction: I-75 SB
From/To: Junction Ent./Livernois Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

Flow Inputs and Adjustments

Volume, V	2630	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	692	v
Trucks and buses	14	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.935	
Driver population factor, fp	1.00	
Flow rate, vp	592	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	592	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	10.8	pc/mi/ln

Level of service, LOS

A

Overall results are not computed when free-flow speed is less than 55 mph.

Phone:
E-mail:

Fax:

----- Operational Analysis -----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: AM Peak
 Freeway/Direction: I-75 SB
 From/To: Livernois Exit/Plaza Ent.
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Flow Inputs and Adjustments -----

Volume, V	2252	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	593	v
Trucks and buses	17	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.922	
Driver population factor, fp	1.00	
Flow rate, vp	643	pc/h/ln

----- Speed Inputs and Adjustments -----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

----- LOS and Performance Measures -----

Flow rate, vp	643	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	11.7	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone:
E-mail:

Fax:

Operational Analysis

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: AM Peak
Freeway/Direction: I-75 SB
From/To: Plaza Ent./Springwells Ent.
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

Flow Inputs and Adjustments

Volume, V	3020	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	795	v
Trucks and buses	24	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.893	
Driver population factor, fp	1.00	
Flow rate, vp	890	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

LOS and Performance Measures

Flow rate, vp	890	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	16.2	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone:
E-mail:

Fax:

-----Operational Analysis-----

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: AM Peak
Freeway/Direction: I-75 SB
From/To: Springwells Ent./Dearborn Ent.
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	3153	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	830	v
Trucks and buses	24	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.893	
Driver population factor, fp	1.00	
Flow rate, vp	929	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	929	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	16.9	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone:
E-mail:

Fax:

----- Operational Analysis -----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: Midday Peak
 Freeway/Direction: I-75 SB
 From/To: Ambassador Ent./Grand Ent.
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Flow Inputs and Adjustments -----

Volume, V	3048	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	802	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	722	pc/h/ln

----- Speed Inputs and Adjustments -----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h

Urban Freeway

----- LOS and Performance Measures -----

Flow rate, vp	722	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	13.1	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone:
E-mail:

Fax:

----- Operational Analysis -----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: Midday Peak
 Freeway/Direction: I-75 SB
 From/To: Grand Ent./Clark Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Flow Inputs and Adjustments -----

Volume, V	3089	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	813	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	610	pc/h/ln

----- Speed Inputs and Adjustments -----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	6	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

----- LOS and Performance Measures -----

Flow rate, vp	610	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	6	
Density, D	11.1	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:
E-mail:

----- Operational Analysis -----

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: Midday Peak
Freeway/Direction: I-75 SB
From/To: Clark Exit/Plaza Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

----- Flow Inputs and Adjustments -----

Volume, V	2791	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	734	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	661	pc/h/ln

----- Speed Inputs and Adjustments -----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

----- LOS and Performance Measures -----

Flow rate, vp	661	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	12.0	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:
E-mail:

----- Operational Analysis -----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: Midday Peak
 Freeway/Direction: I-75 SB
 From/To: Plaza Exit/Junction Ent.
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Flow Inputs and Adjustments -----

Volume, V	2396	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	631	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	709	pc/h/ln

----- Speed Inputs and Adjustments -----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

----- LOS and Performance Measures -----

Flow rate, vp	709	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	12.9	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:
E-mail:

Operational Analysis

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: Midday Peak
Freeway/Direction: I-75 SB
From/To: Junction Ent./Livernois Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

Flow Inputs and Adjustments

Volume, V	2443	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	643	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	579	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h

Urban Freeway

LOS and Performance Measures

Flow rate, vp	579	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	10.5	pc/mi/ln

Level of service, LOS

A

Overall results are not computed when free-flow speed is less than 55 mph.

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E-mail:

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-----Operational Analysis-----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: Midday Peak
 Freeway/Direction: I-75 SB
 From/To: Livernois Exit/Plaza Ent.
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	2273	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	598	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	673	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	673	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	12.2	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:
E-mail:

-----Operational Analysis-----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: Midday Peak
 Freeway/Direction: I-75 SB
 From/To: Plaza Ent./Springwells Ent.
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	2765	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	728	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	819	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	819	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	14.9	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:
E-mail:

Operational Analysis

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: Midday Peak
 Freeway/Direction: I-75 SB
 From/To: Springwells Ent./Dearborn Ent.
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

Flow Inputs and Adjustments

Volume, V	3169	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	834	v
Trucks and buses	25	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.889	
Driver population factor, fp	1.00	
Flow rate, vp	938	pc/h/ln

Speed Inputs and Adjustments

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

LOS and Performance Measures

Flow rate, vp	938	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	17.1	pc/mi/ln

Level of service, LOS

B

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:
E-mail:

----- Operational Analysis -----

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: PM Peak
Freeway/Direction: I-75 SB
From/To: Ambassador Ent./Grand Ent.
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

----- Flow Inputs and Adjustments -----

Volume, V	6041	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1590	v
Trucks and buses	9	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.957	
Driver population factor, fp	1.00	
Flow rate, vp	1329	pc/h/ln

----- Speed Inputs and Adjustments -----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

----- LOS and Performance Measures -----

Flow rate, vp	1329	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	24.2	pc/mi/ln

Level of service, LOS

C

Overall results are not computed when free-flow speed is less than 55 mph.

HCS+: Basic Freeway Segments Release 5.2

Phone: Fax:
E-mail:

----- Operational Analysis -----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: PM Peak
 Freeway/Direction: I-75 SB
 From/To: Grand Ent./Clark Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Flow Inputs and Adjustments -----

Volume, V	6512	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1714	v
Trucks and buses	10	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.952	
Driver population factor, fp	1.00	
Flow rate, vp	1200	pc/h/ln

----- Speed Inputs and Adjustments -----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	6	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

----- LOS and Performance Measures -----

Flow rate, vp	1200	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	6	
Density, D	21.8	pc/mi/ln

Level of service, LOS

C

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:
E-mail:

-----Operational Analysis-----

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: PM Peak
Freeway/Direction: I-75 SB
From/To: Clark Exit/Plaza Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	6228	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1639	v
Trucks and buses	10	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.952	
Driver population factor, fp	1.00	
Flow rate, vp	1377	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	1377	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	25.0	pc/mi/ln

Level of service, LOS

C

Overall results are not computed when free-flow speed is less than 55 mph.

HCS+: Basic Freeway Segments Release 5.2

Phone: Fax:
E-mail:

-----Operational Analysis-----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: PM Peak
 Freeway/Direction: I-75 SB
 From/To: Plaza Exit/Junction Ent.
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	5291	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1392	v
Trucks and buses	9	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.957	
Driver population factor, fp	1.00	
Flow rate, vp	1455	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h

Urban Freeway

-----LOS and Performance Measures-----

Flow rate, vp	1455	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	26.5	pc/mi/ln

Level of service, LOS

D

Overall results are not computed when free-flow speed is less than 55 mph.

Phone:
E-mail:

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-----Operational Analysis-----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: PM Peak
 Freeway/Direction: I-75 SB
 From/To: Junction Ent./Livernois Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	5588	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1471	v
Trucks and buses	9	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.957	
Driver population factor, fp	1.00	
Flow rate, vp	1229	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	5	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	0.0	mi/h
Free-flow speed, FFS	55.0	mi/h

Urban Freeway

-----LOS and Performance Measures-----

Flow rate, vp	1229	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	5	
Density, D	22.3	pc/mi/ln

Level of service, LOS

C

Overall results are not computed when free-flow speed is less than 55 mph.

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-----Operational Analysis-----

Analyst: CH
 Agency or Company: PARSONS
 Date Performed: 8/22/2007
 Analysis Time Period: PM Peak
 Freeway/Direction: I-75 SB
 From/To: Livernois Exit/Plaza Ent.
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	5410	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1424	v
Trucks and buses	8	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.962	
Driver population factor, fp	1.00	
Flow rate, vp	1481	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	1481	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	26.9	pc/mi/ln

Level of service, LOS

D

Overall results are not computed when free-flow speed is less than 55 mph.

Phone: Fax:
E-mail:

-----Operational Analysis-----

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: PM Peak
Freeway/Direction: I-75 SB
From/To: Plaza Ent./Springwells Ent.
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	5900	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1553	v
Trucks and buses	12	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.943	
Driver population factor, fp	1.00	
Flow rate, vp	1646	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h
	Urban Freeway	

-----LOS and Performance Measures-----

Flow rate, vp	1646	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	29.9	pc/mi/ln

Level of service, LOS

D

Overall results are not computed when free-flow speed is less than 55 mph.

HCS+: Basic Freeway Segments Release 5.2

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-----Operational Analysis-----

Analyst: CH
Agency or Company: PARSONS
Date Performed: 8/22/2007
Analysis Time Period: PM Peak
Freeway/Direction: I-75 SB
From/To: Springwells Ent./Dearborn Ent.
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

-----Flow Inputs and Adjustments-----

Volume, V	6247	veh/h
Peak-hour factor, PHF	0.95	
Peak 15-min volume, v15	1644	v
Trucks and buses	12	%
Recreational vehicles	0	%
Terrain type:	Level	
Grade	0.00	%
Segment length	0.00	mi
Trucks and buses PCE, ET	1.5	
Recreational vehicle PCE, ER	1.2	
Heavy vehicle adjustment, fHV	0.943	
Driver population factor, fp	1.00	
Flow rate, vp	1743	pc/h/ln

-----Speed Inputs and Adjustments-----

Lane width	12.0	ft
Right-shoulder lateral clearance	6.0	ft
Interchange density	1.00	interchange/mi
Number of lanes, N	4	
Free-flow speed:	Measured	
FFS or BFFS	55.0	mi/h
Lane width adjustment, fLW	0.0	mi/h
Lateral clearance adjustment, fLC	0.0	mi/h
Interchange density adjustment, fID	2.5	mi/h
Number of lanes adjustment, fN	1.5	mi/h
Free-flow speed, FFS	55.0	mi/h

Urban Freeway

-----LOS and Performance Measures-----

Flow rate, vp	1743	pc/h/ln
Free-flow speed, FFS	55.0	mi/h
Average passenger-car speed, S	55.0	mi/h
Number of lanes, N	4	
Density, D	31.7	pc/mi/ln

Level of service, LOS

D

Overall results are not computed when free-flow speed is less than 55 mph.

Phone:
E-mail:

Fax:

-----Diverge Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/22/2007
 Analysis time period: AM Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Exit Ramp W of Dearborn
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	5240	vph

-----Off Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	99	vph
Length of first accel/decel lane	120	ft
Length of second accel/decel lane		ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	5240	99		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	1379	26		v
Trucks and buses	16	11		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5		1.5	
Recreational vehicle PCE, ER	1.2		1.2	

Heavy vehicle adjustment, fHV	0.926	0.948	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	5957	110	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.436 Using Equation 8
FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 2659$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	5957	9000	No
v_{12}	2659	4400	No
$v_{FO} = v_F - v_R$	5847	9000	No
v_R	110	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 26.0$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence C

----- Speed Estimation -----

Intermediate speed variable,	D = 0.438	
Space mean speed in ramp influence area,	S = 49.3	mph
Space mean speed in outer lanes,	S = 57.8	mph
Space mean speed for all vehicles,	S = 53.7	mph

Phone: Fax:
E-mail:

-----Diverge Analysis-----

Analyst: CH
Agency/Co.: PARSONS
Date performed: 8/22/2007
Analysis time period: AM Peak
Freeway/Dir of Travel: I-75 NB
Junction: Exit Ramp W of Springwells
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	5141	vph

-----Off Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	402	vph
Length of first accel/decel lane	250	ft
Length of second accel/decel lane		ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	5141	402		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	1353	106		v
Trucks and buses	16	2		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	%	-3.31	%
Length	0.00	mi	0.14	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.926	0.990	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	5845	427	pcph

----- Estimation of V12 Diverge Areas -----

$$L = \frac{EQ}{FD} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.436 \quad \text{Using Equation 8}$$

$$v_{12} = v_R + (v_F - v_R) P = 2789 \quad \text{pc/h}$$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	5845	9000	No
v_{12}	2789	4400	No
$v_{FO} = v_F - v_R$	5418	9000	No
v_R	427	2000	No

----- Level of Service Determination (if not F) -----

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 26.0 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence C

----- Speed Estimation -----

Intermediate speed variable,	$D = 0.466$	
Space mean speed in ramp influence area,	$S_R = 48.9$	mph
Space mean speed in outer lanes,	$S_0 = 58.3$	mph
Space mean speed for all vehicles,	$S = 53.4$	mph

Phone:
E-mail:

Fax:

-----Diverge Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/22/2007
 Analysis time period: AM Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Plaza Exit Ramp W of Waterman
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4739	vph

-----Off Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	45.0	mph
Volume on ramp	394	vph
Length of first accel/decel lane	1963	ft
Length of second accel/decel lane	0	ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4739	394		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	1247	104		v
Trucks and buses	17	25		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	%	5.00	%
Length	0.00	mi	0.17	mi
Trucks and buses PCE, ET	1.5		1.5	
Recreational vehicle PCE, ER	1.2		2.5	

Heavy vehicle adjustment, fHV	0.922	0.889	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	5412	467	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.260 Using Equation 0
FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 1753$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	5412	9000	No
v_{12}	1753	4400	No
$v_{FO} = v_F - v_R$	4945	9000	No
v_R	467	4100	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = -16.0$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence A

----- Speed Estimation -----

Intermediate speed variable,	D = 0.340	
Space mean speed in ramp influence area,	S = 50.6	mph
Space mean speed in outer lanes,	S = 57.1	mph
Space mean speed for all vehicles,	S = 54.8	mph

Phone: Fax:
E-mail:

----- Merge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: AM Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Entrance Ramp W of Livernois
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4345	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	307	vph
Length of first accel/decel lane	1164	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4345	307		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	1143	81		v
Trucks and buses	11	3		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	%	-1.20	%	%
Length	mi	0.06	mi	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.948	0.985	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4825	328	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.548 Using Equation 4
FM
 $v_{12} = v_F (P_{FM}) = 2642 \text{ pc/h}$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v FO	5153	9000	No
v R12	2970	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 21.2 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence C

----- Speed Estimation -----

Intermediate speed variable,	M = 0.316	
Space mean speed in ramp influence area,	S _R = 50.9	mph
Space mean speed in outer lanes,	S ₀ = 52.9	mph
Space mean speed for all vehicles,	S = 51.7	mph

Phone: Fax:
 E-mail:

-----Diverge Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: AM Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Exit Ramp W of Junction
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	4652	vph

-----Off Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	142	vph
Length of first accel/decel lane	647	ft
Length of second accel/decel lane		ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	4652	142		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	1224	37		v
Trucks and buses	11	5		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	%	1.28	%
Length	0.00	mi	0.03	mi
Trucks and buses PCE, ET	1.5		1.5	
Recreational vehicle PCE, ER	1.2		1.2	

Heavy vehicle adjustment, fHV	0.948	0.976	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	5166	153	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.436 Using Equation 8
FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 2339$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	5166	9000	No
v_{12}	2339	4400	No
$v_{FO} = v_F - v_R$	5013	9000	No
v_R	153	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 18.5$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable, $D = 0.442$
Space mean speed in ramp influence area, $S_R = 49.3$ mph
Space mean speed in outer lanes, $S_0 = 58.7$ mph
Space mean speed for all vehicles, $S = 54.0$ mph

Phone: Fax:
E-mail:

----- Merge Analysis -----

Analyst: CH
Agency/Co.: PARSONS
Date performed: 8/23/2007
Analysis time period: AM Peak
Freeway/Dir of Travel: I-75 NB
Junction: Entrance Ramp E of Clark
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	5203	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	329	vph
Length of first accel/decel lane	590	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	5203	329		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	1369	87		v
Trucks and buses	13	22		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	%	%	%	%
Length	mi	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.939	0.901	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	5833	384	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.358 Using Equation 4
FM
 $v_{12} = v_F (P_{FM}) = 2087 \text{ pc/h}$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v _{FO}	6217	9000	No
v _{R12}	2471	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 20.9 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence C

----- Speed Estimation -----

Intermediate speed variable,	M = 0.326	
Space mean speed in ramp influence area,	S _R = 50.8	mph
Space mean speed in outer lanes,	S _O = 50.1	mph
Space mean speed for all vehicles,	S _A = 50.3	mph

Phone:
E-mail:

Fax:

-----Diverge Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: AM Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Exit Ramp E of Grand
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Diverge	
Number of lanes in freeway	5	
Free-flow speed on freeway	55.0	mph
Volume on freeway	5532	vph

-----Off Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	20	vph
Length of first accel/decel lane	235	ft
Length of second accel/decel lane		ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	5532	20		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	1456	5		v
Trucks and buses	14	25		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	%	2.94	%
Length	0.00	mi	0.13	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.935	0.889	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	6231	24	pcph

----- Estimation of V12 Diverge Areas -----

$$L = \frac{EQ}{FD} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.436 \quad \text{Using Equation 8}$$

$$v_{12} = v_R + (v_F - v_R) P = 2323 \quad \text{pc/h}$$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	5297	9000	No
v_{12}	2323	4400	No
$v_{FO} = v_F - v_R$	5273	9000	No
v_R	24	2000	No

----- Level of Service Determination (if not F) -----

$$\text{Density, } D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 22.1 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence C

----- Speed Estimation -----

Intermediate speed variable,	$D = 0.430$	
Space mean speed in ramp influence area,	$S_R = 49.4$	mph
Space mean speed in outer lanes,	$S_0 = 58.4$	mph
Space mean speed for all vehicles,	$S = 54.1$	mph

Phone: Fax:
E-mail:

----- Diverge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: Midday Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Exit Ramp W of Dearborn
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2795	vph

----- Off Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	44	vph
Length of first accel/decel lane	120	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2795	44		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	736	12		v
Trucks and buses	25	7		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.889	0.966	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3310	48	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
 EQ
 P = 0.436 Using Equation 8
 FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 1470$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	3310	9000	No
v_{12}	1470	4400	No
$v_{FO} = v_F - v_R$	3262	9000	No
v_R	48	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 15.8$ pc/mi/ln
 Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	D = 0.432	
Space mean speed in ramp influence area,	S = 49.4	mph
Space mean speed in outer lanes,	S = 60.3	mph
Space mean speed for all vehicles,	S = 54.9	mph

Phone:
E-mail:

Fax:

----- Diverge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: Midday Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Exit Ramp W of Springwells
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2751	vph

----- Off Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	273	vph
Length of first accel/decel lane	250	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2751	273		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	724	72		v
Trucks and buses	25	10		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	%	-3.31	%
Length	0.00	mi	0.14	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.889	0.952	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3258	302	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.436 Using Equation 8
FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 1591$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	3258	9000	No
v_{12}	1591	4400	No
$v_{FO} = v_F - v_R$	2956	9000	No
v_R	302	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 15.7$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	D = 0.455	
Space mean speed in ramp influence area,	S = 49.1	mph
Space mean speed in outer lanes,	S = 60.3	mph
Space mean speed for all vehicles,	S = 54.3	mph

Phone:
E-mail:

Fax:

-----Diverge Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: Midday Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Plaza Exit Ramp W of Waterman
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2477	vph

-----Off Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	45.0	mph
Volume on ramp	708	vph
Length of first accel/decel lane	1963	ft
Length of second accel/decel lane	0	ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2477	708		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	652	186		v
Trucks and buses	25	25		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	%	5.00	%
Length	0.00	mi	0.17	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	2.5		

Heavy vehicle adjustment, fHV	0.889	0.889	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2933	838	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.260 Using Equation 0
FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 1383$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2933	9000	No
v_{12}	1383	4400	No
$v_{FO} = v_F - v_R$	2095	9000	No
v_R	838	4100	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = -19.2$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence A

----- Speed Estimation -----

Intermediate speed variable,	D = 0.373	
Space mean speed in ramp influence area,	S = 50.1	mph
Space mean speed in outer lanes,	S = 60.3	mph
Space mean speed for all vehicles,	S = 55.1	mph

Phone: Fax:
 E-mail:

----- Merge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: Midday Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Entrance Ramp W of Livernois
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1769	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	111	vph
Length of first accel/decel lane	1164	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1769	111		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	466	29		v
Trucks and buses	25	10		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade		%	-1.20	%
Length		mi	0.06	mi
Trucks and buses PCE, ET	1.5		1.5	
Recreational vehicle PCE, ER	1.2		1.2	

Heavy vehicle adjustment, fHV	0.889	0.952	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2095	123	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.573 Using Equation 4
FM
 $v_{12} = v_{F} (P_{FM}) = 1201 \text{ pc/h}$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v _{FO}	2218	9000	No
v _{R12}	1324	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 8.4 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence A

----- Speed Estimation -----

Intermediate speed variable,	M = 0.254	
Space mean speed in ramp influence area,	S _R = 51.7	mph
Space mean speed in outer lanes,	S ₀ = 55.0	mph
Space mean speed for all vehicles,	S = 53.0	mph

Phone: Fax:
E-mail:

-----Diverge Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: Midday Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Exit Ramp W of Junction
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1880	vph

-----Off Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	51	vph
Length of first accel/decel lane	647	ft
Length of second accel/decel lane		ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1880	51		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	495	13		v
Trucks and buses	25	18		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	%	1.28	%
Length	0.00	mi	0.03	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.889	0.917	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2226	59	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.436 Using Equation 8
FD
 $v_{12} = v_R + (v_F - v_R) P = 1004$ pc/h
FD

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2226	9000	No
v_{12}	1004	4400	No
$v_{FO} = v_F - v_R$	2167	9000	No
v_R	59	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 7.1$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence A

----- Speed Estimation -----

Intermediate speed variable,	D = 0.433	
Space mean speed in ramp influence area,	S = 49.4	mph
Space mean speed in outer lanes,	S = 60.3	mph
Space mean speed for all vehicles,	S = 54.8	mph

Phone: Fax:
E-mail:

----- Merge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: Midday Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Entrance Ramp E of Clark
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	1984	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	395	vph
Length of first accel/decel lane	590	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	1984	395		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	522	104		v
Trucks and buses	25	23		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	%	%		%
Length	mi	mi		mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.889	0.897	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2349	464	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.348 Using Equation 4
FM
 $v_{12} = v_F (P_{FM}) = 817 \text{ pc/h}$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v _{FO}	2813	9000	No
v _{R12}	1281	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 11.6 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	M = 0.294	
Space mean speed in ramp influence area,	S _R = 51.2	mph
Space mean speed in outer lanes,	S _O = 54.0	mph
Space mean speed for all vehicles,	S _A = 52.7	mph

Phone:
E-mail:

Fax:

----- Diverge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: Midday Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Exit Ramp E of Grand
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Diverge	
Number of lanes in freeway	5	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2379	vph

----- Off Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	244	vph
Length of first accel/decel lane	235	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2379	244		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	626	64		v
Trucks and buses	25	25		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	%	2.94	%
Length	0.00	mi	0.13	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.889	0.889	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2817	289	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.436 Using Equation 8
FD
 $v_{12} = v_R + (v_F - v_R) P = 1391$ pc/h
FD

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2817	9000	No
v_{12}	1391	4400	No
$v_{FO} = v_F - v_R$	2528	9000	No
v_R	289	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 14.1$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	D = 0.454	
Space mean speed in ramp influence area,	S = 49.1	mph
Space mean speed in outer lanes,	S = 60.3	mph
Space mean speed for all vehicles,	S = 54.2	mph

Phone: Fax:
 E-mail:

-----Diverge Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: PM Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Exit Ramp W of Dearborn
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3422	vph

-----Off Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	29	vph
Length of first accel/decel lane	120	ft
Length of second accel/decel lane		ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3422	29		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	901	8		v
Trucks and buses	25	7		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5		1.5	
Recreational vehicle PCE, ER	1.2		1.2	

Heavy vehicle adjustment, fHV	0.889	0.966	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4052	32	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.436 Using Equation 8
FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 1785$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	4052	9000	No
v_{12}	1785	4400	No
$v_{FO} = v_F - v_R$	4020	9000	No
v_R	32	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 18.5$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	D = 0.431	
Space mean speed in ramp influence area,	S = 49.4	mph
Space mean speed in outer lanes,	S = 59.8	mph
Space mean speed for all vehicles,	S = 54.7	mph

Phone:
E-mail:

Fax:

-----Diverge Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: PM Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Exit Ramp W of Springwells
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3391	vph

-----Off Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	277	vph
Length of first accel/decel lane	250	ft
Length of second accel/decel lane		ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3391	277		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	892	73		v
Trucks and buses	25	1		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00 %	-3.31 %		%
Length	0.00 mi	0.14 mi		mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.889	0.995	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	4016	293	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.436 Using Equation 8
FD
 $v_{12} = v_R + (v_F - v_R) P = 1916$ pc/h
FD

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	4016	9000	No
v_{12}	1916	4400	No
$v_{FO} = v_F - v_R$	3723	9000	No
v_R	293	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 18.5$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	D = 0.454	
Space mean speed in ramp influence area,	S = 49.1	mph
Space mean speed in outer lanes,	S = 60.1	mph
Space mean speed for all vehicles,	S = 54.3	mph

Phone:
E-mail:

Fax:

----- Diverge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: PM Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Plaza Exit Ramp W of Waterman
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3116	vph

----- Off Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	45.0	mph
Volume on ramp	969	vph
Length of first accel/decel lane	1963	ft
Length of second accel/decel lane	0	ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3116	969		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	820	255		v
Trucks and buses	25	25		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	%	5.00	%
Length	0.00	mi	0.17	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	2.5		

Heavy vehicle adjustment, fHV	0.889	0.889	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3690	1148	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.260 Using Equation 0
FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 1809 \text{ pc/h}$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	3690	9000	No
v_{12}	1809	4400	No
$v_{FO} = v_F - v_R$	2542	9000	No
v_R	1148	4100	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = -15.5 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence A

----- Speed Estimation -----

Intermediate speed variable,	D = 0.401	
Space mean speed in ramp influence area,	S = 49.8	mph
Space mean speed in outer lanes,	S = 60.3	mph
Space mean speed for all vehicles,	S = 54.7	mph

Phone: Fax:
 E-mail:

----- Merge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: PM Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Entrance Ramp W of Livernois
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2147	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	133	vph
Length of first accel/decel lane	1164	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2147	133		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	565	35		v
Trucks and buses	18	17		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade		%	-1.20	%
Length		mi	0.06	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.917	0.922	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2463	152	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.570 Using Equation 4
FM
 $v_{12} = v_F (P_{FM}) = 1403 \text{ pc/h}$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v _{FO}	2615	9000	No
v _{R12}	1555	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 10.2 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	M = 0.258	
Space mean speed in ramp influence area,	S _S = 51.6	mph
Space mean speed in outer lanes,	S _R = 54.9	mph
Space mean speed for all vehicles,	S _O = 52.9	mph

Phone: Fax:
E-mail:

----- Diverge Analysis -----

Analyst: CH
Agency/Co.: PARSONS
Date performed: 8/23/2007
Analysis time period: PM Peak
Freeway/Dir of Travel: I-75 NB
Junction: Exit Ramp W of Junction
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2280	vph

----- Off Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	61	vph
Length of first accel/decel lane	647	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2280	61		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	600	16		v
Trucks and buses	17	15		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	% 1.28	%	%
Length	0.00	mi 0.03	mi	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.922	0.930	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2604	69	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.436 Using Equation 8
FD
 $v_{12} = v_R + (v_F - v_R) P = 1174$ pc/h
FD

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{12} = v_{Fi}$	2604	9000	No
v_{12}	1174	4400	No
$v_{FO} = v_F - v_R$	2535	9000	No
v_R	69	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 8.5$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence A

----- Speed Estimation -----

Intermediate speed variable,	D = 0.434	
Space mean speed in ramp influence area,	S = 49.4	mph
Space mean speed in outer lanes,	S = 60.3	mph
Space mean speed for all vehicles,	S = 54.8	mph

Phone: _____ Fax: _____
 E-mail: _____

----- Merge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: PM Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Entrance Ramp W of Clark
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2340	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	709	vph
Length of first accel/decel lane	590	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2340	709		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	616	187		v
Trucks and buses	21	13		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	%	%		%
Length	mi	mi		mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.905	0.939	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2722	795	pcph

----- Estimation of V12 Merge Areas -----

$$L = \frac{EQ}{P} \quad (\text{Equation 25-2 or 25-3})$$

$$P = 0.306 \quad \text{Using Equation 4}$$

$$v_{12} = v_{F, FM} (P) = 834 \quad \text{pc/h}$$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v _{FO}	3517	9000	No
v _{R12}	1629	4600	No

----- Level of Service Determination (if not F) -----

$$\text{Density, } D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 14.1 \quad \text{pc/mi/ln}$$

Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	M = 0.300	
Space mean speed in ramp influence area,	S _S = 51.1	mph
Space mean speed in outer lanes,	S _R = 53.4	mph
Space mean speed for all vehicles,	S _O = 52.3	mph

Phone:
E-mail:

Fax:

-----Diverge Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: PM Peak
 Freeway/Dir of Travel: I-75 NB
 Junction: Exit Ramp E of Grand
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Diverge	
Number of lanes in freeway	5	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3049	vph

-----Off Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	204	vph
Length of first accel/decel lane	235	ft
Length of second accel/decel lane		ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3049	204		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	802	54		v
Trucks and buses	19	25		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	%	2.94	%
Length	0.00	mi	0.13	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.913	0.889	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3514	242	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.436 Using Equation 8
FD
 $v_{12} = v_R + (v_F - v_R) P = 1669$ pc/h
FD

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	3514	9000	No
v_{12}	1669	4400	No
$v_{FO} = v_F - v_R$	3272	9000	No
v_R	242	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 16.5$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	D = 0.450	
Space mean speed in ramp influence area,	S = 49.2	mph
Space mean speed in outer lanes,	S = 60.3	mph
Space mean speed for all vehicles,	S = 54.5	mph

Phone: Fax:
E-mail:

----- Merge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: AM Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Service Dr Ent Ramp E of Grand
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3386	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	28	vph
Length of first accel/decel lane	590	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	3386	28	vph
Peak-hour factor, PHF	0.95	0.95	
Peak 15-min volume, v15	891	7	v
Trucks and buses	14	18	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	

Heavy vehicle adjustment, fHV	0.935	0.917	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3814	32	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.402 Using Equation 4
FM
 $v_{12} = v_{F} (P_{FM}) = 1532 \text{ pc/h}$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v FO	3846	9000	No
v R12	1564	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 14.0 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	M = 0.298	
	S	
Space mean speed in ramp influence area,	S = 51.1	mph
	R	
Space mean speed in outer lanes,	S = 52.7	mph
	O	
Space mean speed for all vehicles,	S = 52.0	mph

Phone:
E-mail:

Fax:

----- Diverge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: AM Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Exit Ramp E of Clark
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3414	vph

----- Off Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	602	vph
Length of first accel/decel lane	140	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3414	602		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	898	158		v
Trucks and buses	14	1		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade	0.00	%	0.00	%
Length	0.00	mi	0.00	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.935	0.995	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3845	637	pcph

----- Estimation of V12 Diverge Areas -----

$$L = \frac{EQ}{FD} \quad (\text{Equation 25-8 or 25-9})$$

$$P = 0.436 \quad \text{Using Equation 8}$$

$$v_{12R} = v_{FR} + (v_{FR} - v_{FD}) P = 2036 \quad \text{pc/h}$$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_{F12}$	3845	9000	No
v_{12}	2036	4400	No
$v_{FO} = v_{FR} - v_{R}$	3208	9000	No
v_{R}	637	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 20.5 \quad \text{pc/mi/ln}$

Level of service for ramp-freeway junction areas of influence C

----- Speed Estimation -----

Intermediate speed variable, $D = 0.485$

Space mean speed in ramp influence area, $S_R = 48.7 \quad \text{mph}$

Space mean speed in outer lanes, $S_0 = 60.3 \quad \text{mph}$

Space mean speed for all vehicles, $S = 53.6 \quad \text{mph}$

Phone:
E-mail:

Fax:

----- Diverge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: AM Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Plaza Exit Ramp E of Junction
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2812	vph

----- Off Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	45.0	mph
Volume on ramp	216	vph
Length of first accel/decel lane	1963	ft
Length of second accel/decel lane	0	ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2812	216		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	740	57		v
Trucks and buses	17	25		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	%	1.22	%
Length	0.00	mi	0.46	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.922	0.889	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3212	256	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.260 Using Equation 0
FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 1025$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	3212	9000	No
v_{12}	1025	4400	No
$v_{FO} = v_F - v_R$	2956	9000	No
v_R	256	4100	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = -22.3$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence A

----- Speed Estimation -----

Intermediate speed variable,	D = 0.321	
	S	
Space mean speed in ramp influence area,	S = 50.8	mph
	R	
Space mean speed in outer lanes,	S = 60.0	mph
	0	
Space mean speed for all vehicles,	S = 56.7	mph

Phone: Fax:
E-mail:

----- Merge Analysis -----

Analyst: CH
Agency/Co.: PARSONS
Date performed: 8/23/2007
Analysis time period: AM Peak
Freeway/Dir of Travel: I-75 SB
Junction: Ent. Ramp W of Junction
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2596	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	34	vph
Length of first accel/decel lane	1164	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	2596	34	vph
Peak-hour factor, PHF	0.95	0.95	
Peak 15-min volume, v15	683	9	v
Trucks and buses	14	12	%
Recreational vehicles	0	0	%
Terrain type:	Level	Grade	
Grade		% -4.00	%
Length		mi 0.04	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	

Heavy vehicle adjustment, fHV	0.935	0.943	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2924	38	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.584 Using Equation 4
FM
 $v_{12} = v_F (P_{FM}) = 1707$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v FO	2962	9000	No
v R12	1745	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 11.8$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	M = 0.262	
	S	
Space mean speed in ramp influence area,	S = 51.6	mph
	R	
Space mean speed in outer lanes,	S = 54.6	mph
	O	
Space mean speed for all vehicles,	S = 52.8	mph

Phone: Fax:
E-mail:

----- Diverge Analysis -----

Analyst: CH
Agency/Co.: PARSONS
Date performed: 8/23/2007
Analysis time period: AM Peak
Freeway/Dir of Travel: I-75 SB
Junction: Exit Ramp W of Livernois
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2630	vph

----- Off Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	378	vph
Length of first accel/decel lane	647	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2630	378		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	692	99		v
Trucks and buses	14	0		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	% 5.00	%	%
Length	0.00	mi 0.07	mi	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	2.5		

Heavy vehicle adjustment, fHV	0.935	1.000	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2962	398	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.436 Using Equation 8
FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 1516$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2962	9000	No
v_{12}	1516	4400	No
$v_{FO} = v_F - v_R$	2564	9000	No
v_R	398	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 11.5$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	D = 0.464	
	S	
Space mean speed in ramp influence area,	S = 49.0	mph
	R	
Space mean speed in outer lanes,	S = 60.3	mph
	0	
Space mean speed for all vehicles,	S = 53.9	mph

Phone: Fax:
E-mail:

----- Merge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: AM Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Entrance Ramp W of Springwells
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3020	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	134	vph
Length of first accel/decel lane	370	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	3020	134	vph
Peak-hour factor, PHF	0.95	0.95	
Peak 15-min volume, v15	795	35	v
Trucks and buses	24	4	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade		%	%
Length		mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	

Heavy vehicle adjustment, fHV	0.893	0.980	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3560	144	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.318 Using Equation 4
FM
 $v_{12} = v_F (P_{FM}) = 1131 \text{ pc/h}$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v FO	3704	9000	No
v R12	1275	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 13.0 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	M = 0.309	
Space mean speed in ramp influence area,	S = 51.0	mph
Space mean speed in outer lanes,	S = 52.4	mph
Space mean speed for all vehicles,	S = 51.9	mph

Phone: Fax:
E-mail:

----- Merge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: AM Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Entrance Ramp W of Dearborn
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3153	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	59	vph
Length of first accel/decel lane	400	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3153	59		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	830	16		v
Trucks and buses	24	25		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.893	0.889	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3717	70	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.336 Using Equation 4
FM
 $v_{12} = v_F (P_{FM}) = 1251 \text{ pc/h}$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v _{FO}	3787	9000	No
v _{R12}	1321	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 13.2 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	M = 0.308	
Space mean speed in ramp influence area,	S _R = 51.0	mph
Space mean speed in outer lanes,	S ₀ = 52.4	mph
Space mean speed for all vehicles,	S = 51.9	mph

Phone: Fax:
E-mail:

----- Merge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: Midday Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Service Dr Ent Ramp E of Grand
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3048	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	39	vph
Length of first accel/decel lane	590	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	3048	39	vph
Peak-hour factor, PHF	0.95	0.95	
Peak 15-min volume, v15	802	10	v
Trucks and buses	25	15	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade		%	%
Length		mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	

Heavy vehicle adjustment, fHV	0.889	0.930	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3609	44	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
 EQ
 P = 0.400 Using Equation 4
 FM
 $v_{12} = v_F (P_{FM}) = 1445 \text{ pc/h}$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v FO	3653	9000	No
v R12	1489	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 13.4 \text{ pc/mi/ln}$
 Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	M = 0.297	
	S	
Space mean speed in ramp influence area,	S = 51.1	mph
	R	
Space mean speed in outer lanes,	S = 52.9	mph
	0	
Space mean speed for all vehicles,	S = 52.2	mph

Phone: Fax:
E-mail:

----- Diverge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: Midday Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Exit Ramp E of Clark
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3089	vph

----- Off Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	298	vph
Length of first accel/decel lane	140	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	3089	298	vph
Peak-hour factor, PHF	0.95	0.95	
Peak 15-min volume, v15	813	78	v
Trucks and buses	25	6	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00 %	0.00 %	%
Length	0.00 mi	0.00 mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	

Heavy vehicle adjustment, fHV	0.889	0.971	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3658	323	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.436 Using Equation 8
FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 1777$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	3658	9000	No
v_{12}	1777	4400	No
$v_{FO} = v_F - v_R$	3335	9000	No
v_R	323	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 18.3$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	D = 0.457	
Space mean speed in ramp influence area,	S = 49.1	mph
Space mean speed in outer lanes,	S = 60.3	mph
Space mean speed for all vehicles,	S = 54.3	mph

Phone: Fax:
E-mail:

----- Diverge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: Midday Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Plaza Exit Ramp E of Junction
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2791	vph

----- Off Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	45.0	mph
Volume on ramp	395	vph
Length of first accel/decel lane	1963	ft
Length of second accel/decel lane	0	ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2791	395		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	734	104		v
Trucks and buses	25	25		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	%	1.22	%
Length	0.00	mi	0.46	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.889	0.889	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3305	468	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.260 Using Equation 0
FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 1206$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	3305	9000	No
v_{12}	1206	4400	No
$v_{FO} = v_F - v_R$	2837	9000	No
v_R	468	4100	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = -20.7$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence A

----- Speed Estimation -----

Intermediate speed variable,	D = 0.340	
Space mean speed in ramp influence area,	S = 50.6	mph
Space mean speed in outer lanes,	S = 60.1	mph
Space mean speed for all vehicles,	S = 56.3	mph

Phone: Fax:
 E-mail:

----- Merge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: Midday Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Ent. Ramp W of Junction
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2396	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	48	vph
Length of first accel/decel lane	1164	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2396	48		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	631	13		v
Trucks and buses	25	17		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade		% -4.00	%	%
Length		mi 0.04	mi	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.889	0.922	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2837	55	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.582 Using Equation 4
FM
 $v_{12} = v_F (P_{FM}) = 1650$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v FO	2892	9000	No
v R12	1705	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 11.5$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	M = 0.261	
	S	
Space mean speed in ramp influence area,	S = 51.6	mph
	R	
Space mean speed in outer lanes,	S = 54.7	mph
	O	
Space mean speed for all vehicles,	S = 52.8	mph

Phone: Fax:
E-mail:

----- Diverge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: Midday Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Exit Ramp W of Livernois
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Diverge		
Number of lanes in freeway	4		
Free-flow speed on freeway	55.0	mph	
Volume on freeway	2443	vph	

----- Off Ramp Data -----

Side of freeway	Right		
Number of lanes in ramp	1		
Free-Flow speed on ramp	35.0	mph	
Volume on ramp	172	vph	
Length of first accel/decel lane	647	ft	
Length of second accel/decel lane		ft	

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No		
Volume on adjacent ramp		vph	
Position of adjacent ramp			
Type of adjacent ramp			
Distance to adjacent ramp		ft	

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	2443	172		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	643	45		v
Trucks and buses	25	9		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	%	5.00	%
Length	0.00	mi	0.07	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	2.5		

Heavy vehicle adjustment, fHV	0.889	0.957	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	2893	189	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.436 Using Equation 8
FD
 $v_{12} = v_R + (v_F - v_R) P = 1368$ pc/h
FD

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	2893	9000	No
v_{12}	1368	4400	No
$v_{FO} = v_F - v_R$	2704	9000	No
v_R	189	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 10.2$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	D = 0.445	
	S	
Space mean speed in ramp influence area,	S = 49.2	mph
	R	
Space mean speed in outer lanes,	S = 60.3	mph
	O	
Space mean speed for all vehicles,	S = 54.5	mph

Phone: Fax:
E-mail:

----- Merge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: Midday Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Entrance Ramp W of Springwells
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	2765	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	405	vph
Length of first accel/decel lane	370	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	2765	405	vph
Peak-hour factor, PHF	0.95	0.95	
Peak 15-min volume, v15	728	107	v
Trucks and buses	25	22	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade		%	%
Length		mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	

Heavy vehicle adjustment, fHV	0.889	0.901	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3274	473	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.277 Using Equation 4
FM
 $v_{12} = v_F (P_{FM}) = 905$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v FO	3747	9000	No
v R12	1378	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 13.7$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	M = 0.311	
	S	
Space mean speed in ramp influence area,	S = 51.0	mph
	R	
Space mean speed in outer lanes,	S = 52.5	mph
	0	
Space mean speed for all vehicles,	S = 51.9	mph

Phone: Fax:
 E-mail:

----- Merge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: Midday Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Entrance Ramp W of Dearborn
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	3169	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	42	vph
Length of first accel/decel lane	400	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	3169	42		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	834	11		v
Trucks and buses	25	24		%
Recreational vehicles	0	0		%
Terrain type:	Level	Level		
Grade		%	%	%
Length		mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.889	0.893	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	3753	50	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.339 Using Equation 4
FM
 $v_{12} = v_F (P_{FM}) = 1272 \text{ pc/h}$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v _{FO}	3803	9000	No
v _{R12}	1322	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 13.3 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence B

----- Speed Estimation -----

Intermediate speed variable,	M = 0.308	
	S	
Space mean speed in ramp influence area,	S = 51.0	mph
	R	
Space mean speed in outer lanes,	S = 52.3	mph
	0	
Space mean speed for all vehicles,	S = 51.9	mph

Phone: Fax:
E-mail:

----- Merge Analysis -----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: PM Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Service Dr Ent Ramp E of Grand
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

----- Freeway Data -----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	6041	vph

----- On Ramp Data -----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	469	vph
Length of first accel/decel lane	590	ft
Length of second accel/decel lane		ft

----- Adjacent Ramp Data (if one exists) -----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

----- Conversion to pc/h Under Base Conditions -----

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	6041	469	vph
Peak-hour factor, PHF	0.95	0.95	
Peak 15-min volume, v15	1590	123	v
Trucks and buses	9	21	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	0.905	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	6645	546	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.338 Using Equation 4
FM
 $v_{12} = v_F (P_{FM}) = 2243 \text{ pc/h}$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v _{FO}	7191	9000	No
v _{R12}	2789	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 23.3 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence C

----- Speed Estimation -----

Intermediate speed variable,	M = 0.343	
Space mean speed in ramp influence area,	S _R = 50.5	mph
Space mean speed in outer lanes,	S _O = 48.9	mph
Space mean speed for all vehicles,	S = 49.5	mph

Phone: Fax:
E-mail:

-----Diverge Analysis-----

Analyst: CH
Agency/Co.: PARSONS
Date performed: 8/23/2007
Analysis time period: PM Peak
Freeway/Dir of Travel: I-75 SB
Junction: Exit Ramp E of Clark
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	6512	vph

-----Off Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	284	vph
Length of first accel/decel lane	140	ft
Length of second accel/decel lane		ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	6512	284	vph
Peak-hour factor, PHF	0.95	0.95	
Peak 15-min volume, v15	1714	75	v
Trucks and buses	10	10	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	0.00	%	0.00 %
Length	0.00	mi	0.00 mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	

Heavy vehicle adjustment, fHV	0.952	0.952	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	7197	314	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
EQ
P = 0.436 Using Equation 8
FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 3315 \text{ pc/h}$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	7197	9000	No
v_{12}	3315	4400	No
$v_{FO} = v_F - v_R$	6883	9000	No
v_R	314	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 31.5 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence D

----- Speed Estimation -----

Intermediate speed variable,	D = 0.456	
	S	
Space mean speed in ramp influence area,	S = 49.1	mph
	R	
Space mean speed in outer lanes,	S = 56.7	mph
	0	
Space mean speed for all vehicles,	S = 52.9	mph

Phone: Fax:
E-mail:

-----Diverge Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: PM Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Plaza Exit Ramp E of Junction
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	6228	vph

-----Off Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	2	
Free-Flow speed on ramp	45.0	mph
Volume on ramp	937	vph
Length of first accel/decel lane	1963	ft
Length of second accel/decel lane	0	ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp	
Volume, V (vph)	6228	937		vph
Peak-hour factor, PHF	0.95	0.95		
Peak 15-min volume, v15	1639	247		v
Trucks and buses	10	16		%
Recreational vehicles	0	0		%
Terrain type:	Level	Grade		
Grade	0.00	%	1.22	%
Length	0.00	mi	0.46	mi
Trucks and buses PCE, ET	1.5	1.5		
Recreational vehicle PCE, ER	1.2	1.2		

Heavy vehicle adjustment, fHV	0.952	0.926	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	6884	1065	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
 EQ
 P = 0.260 Using Equation 0
 FD
 $v_{12} = v_R + (v_F - v_R) P_{FD} = 2578$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{Fi} = v_F$	6884	9000	No
v_{12}	2578	4400	No
$v_{FO} = v_F - v_R$	5819	9000	No
v_R	1065	4100	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = -8.9$ pc/mi/ln
 Level of service for ramp-freeway junction areas of influence A

----- Speed Estimation -----

Intermediate speed variable,	D = 0.394	
Space mean speed in ramp influence area,	S _R = 49.9	mph
Space mean speed in outer lanes,	S ₀ = 55.8	mph
Space mean speed for all vehicles,	S = 53.4	mph

Phone: Fax:
E-mail:

-----Merge Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: PM Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Ent. Ramp W of Junction
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	5292	vph

-----On Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	296	vph
Length of first accel/decel lane	1164	ft
Length of second accel/decel lane		ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	5292	296	vph
Peak-hour factor, PHF	0.95	0.95	
Peak 15-min volume, v15	1393	78	v
Trucks and buses	9	0	%
Recreational vehicles	0	0	%
Terrain type:	Level	Grade	
Grade		% -4.00	%
Length		mi 0.04	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	

Heavy vehicle adjustment, fHV	0.957	1.000	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	5821	312	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.550 Using Equation 4
FM
 $v_{12} = v_F (P_{FM}) = 3199$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v FO	6133	9000	No
v R12	3511	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 25.4$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence C

----- Speed Estimation -----

Intermediate speed variable,	M = 0.370	
Space mean speed in ramp influence area,	S = 50.2	mph
Space mean speed in outer lanes,	S = 52.1	mph
Space mean speed for all vehicles,	S = 51.0	mph

Phone: Fax:
E-mail:

-----Diverge Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: PM Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Exit Ramp W of Livernois
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	5588	vph

-----Off Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	35.0	mph
Volume on ramp	177	vph
Length of first accel/decel lane	647	ft
Length of second accel/decel lane		ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent ramp		vph
Position of adjacent ramp		
Type of adjacent ramp		
Distance to adjacent ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	5588	177	vph
Peak-hour factor, PHF	0.95	0.95	
Peak 15-min volume, v15	1471	47	v
Trucks and buses	9	23	%
Recreational vehicles	0	0	%
Terrain type:	Level	Grade	
Grade	0.00	%	5.00
Length	0.00	mi	0.07
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	2.5	

Heavy vehicle adjustment, fHV	0.957	0.897	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	6147	208	pcph

----- Estimation of V12 Diverge Areas -----

L = (Equation 25-8 or 25-9)
 EQ
 P = 0.436 Using Equation 8
 FD
 $v_{12} = v_R + (v_F - v_R) P = 2797$ pc/h
 FD

----- Capacity Checks -----

	Actual	Maximum	LOS F?
$v_{12} = v_F$	6147	9000	No
v_{12}	2797	4400	No
$v_{FO} = v_F - v_R$	5939	9000	No
v_R	208	2000	No

----- Level of Service Determination (if not F) -----

Density, $D = 4.252 + 0.0086 v_{12} - 0.009 L_D = 22.5$ pc/mi/ln
 Level of service for ramp-freeway junction areas of influence C

----- Speed Estimation -----

Intermediate speed variable,	D = 0.447	
Space mean speed in ramp influence area,	S = 49.2	mph
Space mean speed in outer lanes,	S = 57.7	mph
Space mean speed for all vehicles,	S = 53.5	mph

Phone: Fax:
E-mail:

-----Merge Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: PM Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Entrance Ramp W of Springwells
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	5900	vph

-----On Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	346	vph
Length of first accel/decel lane	370	ft
Length of second accel/decel lane		ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	5900	346	vph
Peak-hour factor, PHF	0.95	0.95	
Peak 15-min volume, v15	1553	91	v
Trucks and buses	12	16	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	

Heavy vehicle adjustment, fHV	0.943	0.926	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	6583	393	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.287 Using Equation 4
FM
 $v_{12} = v_F (P_{FM}) = 1886 \text{ pc/h}$

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v FO	6976	9000	No
v R12	2279	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 20.8 \text{ pc/mi/ln}$
Level of service for ramp-freeway junction areas of influence C

----- Speed Estimation -----

Intermediate speed variable,	M = 0.333	
Space mean speed in ramp influence area,	S = 50.7	mph
Space mean speed in outer lanes,	S = 48.2	mph
Space mean speed for all vehicles,	S = 49.0	mph

Phone: Fax:
E-mail:

-----Merge Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date performed: 8/23/2007
 Analysis time period: PM Peak
 Freeway/Dir of Travel: I-75 SB
 Junction: Entrance Ramp W of Dearborn
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Freeway Data-----

Type of analysis	Merge	
Number of lanes in freeway	4	
Free-flow speed on freeway	55.0	mph
Volume on freeway	6247	vph

-----On Ramp Data-----

Side of freeway	Right	
Number of lanes in ramp	1	
Free-flow speed on ramp	35.0	mph
Volume on ramp	82	vph
Length of first accel/decel lane	400	ft
Length of second accel/decel lane		ft

-----Adjacent Ramp Data (if one exists)-----

Does adjacent ramp exist?	No	
Volume on adjacent Ramp		vph
Position of adjacent Ramp		
Type of adjacent Ramp		
Distance to adjacent Ramp		ft

-----Conversion to pc/h Under Base Conditions-----

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	6247	82	vph
Peak-hour factor, PHF	0.95	0.95	
Peak 15-min volume, v15	1644	22	v
Trucks and buses	12	4	%
Recreational vehicles	0	0	%
Terrain type:	Level	Level	
Grade	%	%	%
Length	mi	mi	mi
Trucks and buses PCE, ET	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	

Heavy vehicle adjustment, fHV	0.943	0.980	
Driver population factor, fP	1.00	1.00	
Flow rate, vp	6970	88	pcph

----- Estimation of V12 Merge Areas -----

L = (Equation 25-2 or 25-3)
EQ
P = 0.334 Using Equation 4
FM
 $v_{12} = v_F (P_{FM}) = 2330$ pc/h

----- Capacity Checks -----

	Actual	Maximum	LOS F?
v FO	7058	9000	No
v R12	2418	4600	No

----- Level of Service Determination (if not F) -----

Density, $D = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A = 21.8$ pc/mi/ln
Level of service for ramp-freeway junction areas of influence C

----- Speed Estimation -----

Intermediate speed variable,	M = 0.337	
Space mean speed in ramp influence area,	S = 50.6	mph
Space mean speed in outer lanes,	S = 48.3	mph
Space mean speed for all vehicles,	S = 49.1	mph

Phone: Fax:
E-mail:

Operational Analysis

Analyst: CH
Agency/Co.: PARSONS
Date Performed: 8/23/2007
Analysis Time Period: AM Peak
Freeway/Dir of Travel: I-75 NB
Weaving Location: From Livernois Ent/Junct. Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

Inputs

Freeway free-flow speed, SFF	55	mph
Weaving number of lanes, N	5	
Weaving segment length, L	975	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.09	
Weaving ratio, R	0.32	

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	4203	0	142	307	veh/h
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	
Peak 15-min volume, v15	1106	0	37	81	v
Trucks and buses	12	0	5	3	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.943	1.000	0.976	0.985	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	4689	0	153	328	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	0.62	0.24
Weaving and non-weaving speeds, Si	42.74	51.36
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	0.92
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Unconstrained

-----Weaving Segment Speed, Density, Level of Service and Capacity-----

Weaving segment speed, S	50.42	mph
Weaving segment density, D	20.51	pc/mi/ln
Level of service, LOS	C	
Capacity of base condition, cb	9335	pc/h
Capacity as a 15-minute flow rate, c	8807	pc/h
Capacity as a full-hour volume, ch	8367	pc/h

-----Limitations on Weaving Segments-----

	Analyzed	If Max Exceeded	See Note
		Maximum	Note
Weaving flow rate, Vw	481	2800	a
Average flow rate (pcphpl)	1034	2250	b
Volume ratio, VR	0.09	0.20	c
Weaving ratio, R	0.32	N/A	d
Weaving length (ft)	975	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

Phone:
E-mail:

Fax:

Operational Analysis

Analyst: CH
Agency/Co.: PARSONS
Date Performed: 8/23/2007
Analysis Time Period: AM Peak
Freeway/Dir of Travel: I-75 NB
Weaving Location: From Clark Ent. to Grand Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

Inputs

Freeway free-flow speed, SFF	55	mph
Weaving number of lanes, N	5	
Weaving segment length, L	1251	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.07	
Weaving ratio, R	0.06	

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	5184	0	20	329	veh/h
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	
Peak 15-min volume, v15	1364	0	5	87	v
Trucks and buses	13	0	25	22	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.939	1.000	0.889	0.901	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	5811	0	23	384	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	0.58	0.23
Weaving and non-weaving speeds, Si	43.55	51.70
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	0.79
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Unconstrained

_____Weaving Segment Speed, Density, Level of Service and Capacity_____

Weaving segment speed, S	51.07	mph
Weaving segment density, D	24.35	pc/mi/ln
Level of service, LOS	C	
Capacity of base condition, cb	9641	pc/h
Capacity as a 15-minute flow rate, c	9053	pc/h
Capacity as a full-hour volume, ch	8600	pc/h

_____Limitations on Weaving Segments_____

	Analyzed	If Max Exceeded	See Note
Weaving flow rate, Vw	407	2800	a
Average flow rate (pcphpl)	1243	2250	b
Volume ratio, VR	0.07	0.20	c
Weaving ratio, R	0.06	N/A	d
Weaving length (ft)	1251	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

Phone:
E-mail:

Fax:

Operational Analysis

Analyst: CH
Agency/Co.: PARSONS
Date Performed: 8/23/2007
Analysis Time Period: Midday Peak
Freeway/Dir of Travel: I-75 NB
Weaving Location: From Livernois Ent/Junct. Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

Inputs

Freeway free-flow speed, SFF	55	mph
Weaving number of lanes, N	5	
Weaving segment length, L	975	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.08	
Weaving ratio, R	0.32	

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	1719	0	51	111	veh/h
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	
Peak 15-min volume, v15	452	0	13	29	v
Trucks and buses	25	0	18	10	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.889	1.000	0.917	0.952	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	2035	0	58	122	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	0.27	0.08
Weaving and non-weaving speeds, Si	50.52	56.84
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	0.79
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Unconstrained

_____ Weaving Segment Speed, Density, Level of Service and Capacity _____

Weaving segment speed, S	56.27	mph
Weaving segment density, D	7.87	pc/mi/ln
Level of service, LOS	A	
Capacity of base condition, cb	9335	pc/h
Capacity as a 15-minute flow rate, c	8298	pc/h
Capacity as a full-hour volume, ch	7883	pc/h

_____ Limitations on Weaving Segments _____

	Analyzed	If Max Exceeded	See Note
		Maximum	Note
Weaving flow rate, Vw	180	2800	a
Average flow rate (pcphpl)	443	2250	b
Volume ratio, VR	0.08	0.20	c
Weaving ratio, R	0.32	N/A	d
Weaving length (ft)	975	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

Phone: Fax:
E-mail:

-----Operational Analysis-----

Analyst: CH
Agency/Co.: PARSONS
Date Performed: 8/23/2007
Analysis Time Period: Midday Peak
Freeway/Dir of Travel: I-75 NB
Weaving Location: From Clark Ent. to Grand Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

-----Inputs-----

Freeway free-flow speed, SFF	55	mph
Weaving number of lanes, N	5	
Weaving segment length, L	1251	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.27	
Weaving ratio, R	0.38	

-----Conversion to pc/h Under Base Conditions-----

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	1740	0	244	395	veh/h
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	
Peak 15-min volume, v15	458	0	64	104	v
Trucks and buses	25	0	25	23	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.889	1.000	0.889	0.897	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	2060	0	288	463	pc/h

-----Weaving and Non-Weaving Speeds-----

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	0.91	0.09
Weaving and non-weaving speeds, Si	38.54	56.20
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	1.71
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Constrained

_____ Weaving Segment Speed, Density, Level of Service and Capacity _____

Weaving segment speed, S	50.07	mph
Weaving segment density, D	11.23	pc/mi/ln
Level of service, LOS	B	
Capacity of base condition, cb	9061	pc/h
Capacity as a 15-minute flow rate, c	8054	pc/h
Capacity as a full-hour volume, ch	7651	pc/h

_____ Limitations on Weaving Segments _____

	Analyzed	If Max Exceeded	See Note
Weaving flow rate, Vw	751	2800	a
Average flow rate (pcphpl)	562	2250	b
Volume ratio, VR	0.27	0.20	c
Weaving ratio, R	0.38	N/A	d
Weaving length (ft)	1251	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

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Operational Analysis

Analyst: CH
Agency/Co.: PARSONS
Date Performed: 8/23/2007
Analysis Time Period: PM Peak
Freeway/Dir of Travel: I-75 NB
Weaving Location: From Livernois Ent/Junct. Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

Inputs

Freeway free-flow speed, SFF	55	mph
Weaving number of lanes, N	5	
Weaving segment length, L	975	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.08	
Weaving ratio, R	0.31	

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	2085	0	61	133	veh/h
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	
Peak 15-min volume, v15	549	0	16	35	v
Trucks and buses	18	0	15	17	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.917	1.000	0.930	0.922	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	2392	0	69	151	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	0.32	0.09
Weaving and non-weaving speeds, Si	49.21	56.11
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	0.82
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Unconstrained

_____ Weaving Segment Speed, Density, Level of Service and Capacity _____

Weaving segment speed, S	55.45	mph
Weaving segment density, D	9.42	pc/mi/ln
Level of service, LOS	A	
Capacity of base condition, cb	9335	pc/h
Capacity as a 15-minute flow rate, c	8564	pc/h
Capacity as a full-hour volume, ch	8136	pc/h

_____ Limitations on Weaving Segments _____

	Analyzed	If Max Exceeded	See Note
		Maximum	Note
Weaving flow rate, Vw	220	2800	a
Average flow rate (pcphpl)	522	2250	b
Volume ratio, VR	0.08	0.20	c
Weaving ratio, R	0.31	N/A	d
Weaving length (ft)	975	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

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Operational Analysis

Analyst: CH
Agency/Co.: PARSONS
Date Performed: 8/23/2007
Analysis Time Period: PM Peak
Freeway/Dir of Travel: I-75 NB
Weaving Location: From Clark Ent. to Grand Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

Inputs

Freeway free-flow speed, SFF	55	mph
Weaving number of lanes, N	5	
Weaving segment length, L	1251	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.30	
Weaving ratio, R	0.23	

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	2136	0	204	709	veh/h
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	
Peak 15-min volume, v15	562	0	54	187	v
Trucks and buses	18	0	25	13	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.917	1.000	0.889	0.939	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	2450	0	241	794	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	1.18	0.13
Weaving and non-weaving speeds, Si	35.62	54.69
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	1.85
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Constrained

_____ Weaving Segment Speed, Density, Level of Service and Capacity _____

Weaving segment speed, S	47.19	mph
Weaving segment density, D	14.77	pc/mi/ln
Level of service, LOS	B	
Capacity of base condition, cb	9061	pc/h
Capacity as a 15-minute flow rate, c	8313	pc/h
Capacity as a full-hour volume, ch	7897	pc/h

_____ Limitations on Weaving Segments _____

	Analyzed	If Max Exceeded	See Note
		Maximum	Note
Weaving flow rate, Vw	1035	2800	a
Average flow rate (pcphpl)	697	2250	b
Volume ratio, VR	0.30	0.20	c
Weaving ratio, R	0.23	N/A	d
Weaving length (ft)	1251	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

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Operational Analysis

Analyst: CH
 Agency/Co.: PARSONS
 Date Performed: 8/23/2007
 Analysis Time Period: AM Peak
 Freeway/Dir of Travel: I-75 SB
 Weaving Location: From Amb. Ent. to Clark Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

Inputs

Freeway free-flow speed, SFF 55 mph
 Weaving number of lanes, N 5
 Weaving segment length, L 1316 ft
 Terrain type Level
 Grade %
 Length mi
 Weaving type A
 Volume ratio, VR 0.20
 Weaving ratio, R 0.17

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	2692	0	602	120	veh/h
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	
Peak 15-min volume, v15	708	0	158	32	v
Trucks and buses	17	0	1	4	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.922	1.000	0.995	0.980	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	3074	0	636	128	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	1.05	0.11
Weaving and non-weaving speeds, Si	36.96	55.67
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	1.48
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Constrained

_____ Weaving Segment Speed, Density, Level of Service and Capacity _____

Weaving segment speed, S	50.57	mph
Weaving segment density, D	15.18	pc/mi/ln
Level of service, LOS	B	
Capacity of base condition, cb	9134	pc/h
Capacity as a 15-minute flow rate, c	8418	pc/h
Capacity as a full-hour volume, ch	7997	pc/h

_____ Limitations on Weaving Segments _____

	Analyzed	If Max Exceeded Maximum	See Note Note
Weaving flow rate, Vw	764	2800	a
Average flow rate (pcphpl)	767	2250	b
Volume ratio, VR	0.20	0.20	c
Weaving ratio, R	0.17	N/A	d
Weaving length (ft)	1316	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

Phone:
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Operational Analysis

Analyst: CH
Agency/Co.: PARSONS
Date Performed: 8/23/2007
Analysis Time Period: AM Peak
Freeway/Dir of Travel: I-75 SB
Weaving Location: From Junct. Ent. to Liver.Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

Inputs

Freeway free-flow speed, SFF	55	mph
Weaving number of lanes, N	5	
Weaving segment length, L	1100	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.15	
Weaving ratio, R	0.09	

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	2218	0	378	34	veh/h
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	
Peak 15-min volume, v15	584	0	99	9	v
Trucks and buses	17	0	0	12	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.922	1.000	1.000	0.943	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	2533	0	397	37	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	0.37	0.13
Weaving and non-weaving speeds, Si	47.94	54.91
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	1.17
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Unconstrained

_____Weaving Segment Speed, Density, Level of Service and Capacity_____

Weaving segment speed, S	53.77	mph
Weaving segment density, D	11.04	pc/mi/ln
Level of service, LOS	B	
Capacity of base condition, cb	9216	pc/h
Capacity as a 15-minute flow rate, c	8494	pc/h
Capacity as a full-hour volume, ch	8069	pc/h

_____Limitations on Weaving Segments_____

	Analyzed	If Max Exceeded	See Note
		Maximum	Note
Weaving flow rate, Vw	434	2800	a
Average flow rate (pcphpl)	593	2250	b
Volume ratio, VR	0.15	0.20	c
Weaving ratio, R	0.09	N/A	d
Weaving length (ft)	1100	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

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Operational Analysis

Analyst: CH
Agency/Co.: PARSONS
Date Performed: 8/23/2007
Analysis Time Period: Midday Peak
Freeway/Dir of Travel: I-75 SB
Weaving Location: From Amb. Ent. to Clark Exit
Jurisdiction:
Analysis Year: 2035 (PA02)
Description: Detroit River International Crossing Project

Inputs

Freeway free-flow speed, SFF	55	mph
Weaving number of lanes, N	5	
Weaving segment length, L	1316	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.12	
Weaving ratio, R	0.23	

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	2702	0	298	89	veh/h
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	
Peak 15-min volume, v15	711	0	78	23	v
Trucks and buses	25	0	6	7	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.889	1.000	0.971	0.966	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	3199	0	323	96	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	0.36	0.13
Weaving and non-weaving speeds, Si	48.03	54.84
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	1.06
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Unconstrained

_____Weaving Segment Speed, Density, Level of Service and Capacity_____

Weaving segment speed, S	53.95	mph
Weaving segment density, D	13.41	pc/mi/ln
Level of service, LOS	B	
Capacity of base condition, cb	9617	pc/h
Capacity as a 15-minute flow rate, c	8548	pc/h
Capacity as a full-hour volume, ch	8121	pc/h

_____Limitations on Weaving Segments_____

	Analyzed	If Max Exceeded	See Note
Weaving flow rate, Vw	419	2800	a
Average flow rate (pcphpl)	723	2250	b
Volume ratio, VR	0.12	0.20	c
Weaving ratio, R	0.23	N/A	d
Weaving length (ft)	1316	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

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Operational Analysis

Analyst: CH
 Agency/Co.: PARSONS
 Date Performed: 8/23/2007
 Analysis Time Period: Midday Peak
 Freeway/Dir of Travel: I-75 SB
 Weaving Location: From Junct. Ent. to Liver.Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

Inputs

Freeway free-flow speed, SFF 55 mph
 Weaving number of lanes, N 5
 Weaving segment length, L 1100 ft
 Terrain type Level
 Grade %
 Length mi
 Weaving type A
 Volume ratio, VR 0.08
 Weaving ratio, R 0.22

Conversion to pc/h Under Base Conditions

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	2225	0	172	48	veh/h
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	
Peak 15-min volume, v15	586	0	45	13	v
Trucks and buses	25	0	9	17	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.889	1.000	0.957	0.922	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	2634	0	189	54	pc/h

Weaving and Non-Weaving Speeds

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	0.31	0.10
Weaving and non-weaving speeds, Si	49.23	55.98
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7) 0.84
 Maximum number of lanes, Nw (max) (Exhibit 24-7) 1.40
 Type of operation is Unconstrained

_____Weaving Segment Speed, Density, Level of Service and Capacity_____

Weaving segment speed, S 55.34 mph
 Weaving segment density, D 10.40 pc/mi/ln
 Level of service, LOS B
 Capacity of base condition, cb 9484 pc/h
 Capacity as a 15-minute flow rate, c 8430 pc/h
 Capacity as a full-hour volume, ch 8008 pc/h

_____Limitations on Weaving Segments_____

	Analyzed	If Max Exceeded	See Note
Weaving flow rate, Vw	243	2800	a
Average flow rate (pcphpl)	575	2250	b
Volume ratio, VR	0.08	0.20	c
Weaving ratio, R	0.22	N/A	d
Weaving length (ft)	1100	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

HCS+: Freeway Weaving Release 5.2

Phone:
E-mail:

Fax:

-----Operational Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date Performed: 8/23/2007
 Analysis Time Period: PM Peak
 Freeway/Dir of Travel: I-75 SB
 Weaving Location: From Amb. Ent. to Clark Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Inputs-----

Freeway free-flow speed, SFF	55	mph
Weaving number of lanes, N	5	
Weaving segment length, L	1316	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.14	
Weaving ratio, R	0.32	

-----Conversion to pc/h Under Base Conditions-----

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	5648	0	284	580	veh/h
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	
Peak 15-min volume, v15	1486	0	75	153	v
Trucks and buses	9	0	10	22	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.957	1.000	0.952	0.901	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	6212	0	313	677	pc/h

-----Weaving and Non-Weaving Speeds-----

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	0.74	0.34
Weaving and non-weaving speeds, Si	40.91	48.52
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	1.26
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Unconstrained

-----Weaving Segment Speed, Density, Level of Service and Capacity-----

Weaving segment speed, S	47.31	mph
Weaving segment density, D	30.44	pc/mi/ln
Level of service, LOS	D	
Capacity of base condition, cb	9491	pc/h
Capacity as a 15-minute flow rate, c	9082	pc/h
Capacity as a full-hour volume, ch	8628	pc/h

-----Limitations on Weaving Segments-----

	Analyzed	If Max Exceeded	See Note
Weaving flow rate, Vw	990	2800	a
Average flow rate (pcphpl)	1440	2250	b
Volume ratio, VR	0.14	0.20	c
Weaving ratio, R	0.32	N/A	d
Weaving length (ft)	1316	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

HCS+: Freeway Weaving Release 5.2

Phone:
E-mail:

Fax:

-----Operational Analysis-----

Analyst: CH
 Agency/Co.: PARSONS
 Date Performed: 8/23/2007
 Analysis Time Period: PM Peak
 Freeway/Dir of Travel: I-75 SB
 Weaving Location: From Junct. Ent. to Liver.Exit
 Jurisdiction:
 Analysis Year: 2035 (PA02)
 Description: Detroit River International Crossing Project

-----Inputs-----

Freeway free-flow speed, SFF	55	mph
Weaving number of lanes, N	5	
Weaving segment length, L	1100	ft
Terrain type	Level	
Grade		%
Length		mi
Weaving type	A	
Volume ratio, VR	0.08	
Weaving ratio, R	0.40	

-----Conversion to pc/h Under Base Conditions-----

	Non-Weaving		Weaving		
	V	V	V	V	
	A-C	B-D	A-D	B-C	
Volume, V	5114	0	177	296	veh/h
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	
Peak 15-min volume, v15	1346	0	47	78	v
Trucks and buses	9	0	23	0	%
Recreational vehicles	0	0	0	0	%
Trucks and buses PCE, ET	1.5	1.5	1.5	1.5	
Recreational vehicle PCE, ER	1.2	1.2	1.2	1.2	
Heavy vehicle adjustment, fHV	0.957	1.000	0.897	1.000	
Driver population adjustment, fP	1.00	1.00	1.00	1.00	
Flow rate, v	5625	0	207	311	pc/h

-----Weaving and Non-Weaving Speeds-----

	Weaving	Non-Weaving
a (Exhibit 24-6)	0.15	0.00
b (Exhibit 24-6)	2.20	4.00
c (Exhibit 24-6)	0.97	1.30
d (Exhibit 24-6)	0.80	0.75
Weaving intensity factor, Wi	0.66	0.26
Weaving and non-weaving speeds, Si	42.17	50.63
Number of lanes required for		

unconstrained operation, Nw (Exhibit 24-7)	0.90
Maximum number of lanes, Nw (max) (Exhibit 24-7)	1.40
Type of operation is	Unconstrained

_____ Weaving Segment Speed, Density, Level of Service and Capacity _____

Weaving segment speed, S	49.79	mph
Weaving segment density, D	24.68	pc/mi/ln
Level of service, LOS	C	
Capacity of base condition, cb	9484	pc/h
Capacity as a 15-minute flow rate, c	9076	pc/h
Capacity as a full-hour volume, ch	8622	pc/h

_____ Limitations on Weaving Segments _____

	Analyzed	If Max Exceeded	See Note
Weaving flow rate, Vw	518	2800	a
Average flow rate (pcphpl)	1228	2250	b
Volume ratio, VR	0.08	0.20	c
Weaving ratio, R	0.40	N/A	d
Weaving length (ft)	1100	2500	e

Notes:

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- c. Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- i. Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.